

**PREPARED FOR:**



# **GEOTECHNICAL REPORT**

## **PLATTEVILLE GF2 GRANT COUNTY, WISCONSIN**

**EDGE PROJECT NUMBER: 31990**

**April 15, 2022**



**Edge**  
Consulting Engineers, Inc.

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**Reliable  
Comprehensive  
Exceeding Expectations**

# GEOTECHNICAL REPORT

**Project Name:** Platteville GF2

**Site Address:** 5657 C.T.H. "D"  
Platteville, Wisconsin 53813

**Site Coordinates:** 42° 42' 33.98" (42.709358) N Latitude  
90° 29' 08.71" (90.485753) W Longitude

**Client:** Grant County  
111 S. Jefferson St.  
Lancaster, WI 53813-0529  
Contact: Sheriff Nate Dreckman  
Phone: 608.723.2157

**Consultant:** Edge Consulting Engineers, Inc.  
624 Water Street  
Prairie du Sac, Wisconsin 53578  
Contact: Arlen Ostreng, P.E.  
Phone: 608.644.1449

**Edge Project Number:** 31990

**Date:** April 15, 2022

  
Kaitlin Rinabarger  
Geotechnical Specialist

4/15/2022  
Date

  
Arlen Ostreng, P.E.  
Geotechnical Manager

4/15/2022  
Date



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# **SECTION 1**

## **INTRODUCTION**

### **1.1 PROJECT INFORMATION**

This report summarizes the results of a geotechnical exploration conducted by Edge Consulting Engineers, Inc. (Edge Consulting) for US Cellular (USCC), who is proposing to construct a new 255-foot self-support telecommunications tower located at 5657 C.T.H. "D" in the Town of Platteville in Grant County, Wisconsin. Proposed equipment will be located near the base of the proposed tower within a fenced compound. A street map showing the location of the proposed tower site is available in Figure 1. The location of the proposed project site on the Platteville, Wisconsin United States Geological Survey (USGS) 7.5 Minute Quadrangle is shown in Figure 2. Based on an inspection of the quadrangle map and detailed site survey, the site is located at an approximate elevation of 965 feet above mean sea level. A site plan depicting the proposed project has been included as Figure 3. Site photos have also been included in Figure 4.

### **1.2 PURPOSE OF REPORT**

The investigative activities of this report were conducted for the purposes of providing geotechnical engineering design parameters, soil characteristics, tower and equipment structure foundation recommendations, and site development recommendations with respect to the proposed improvements. This assessment was completed in conformance with client directed protocols, and utilizing the judgment of the geotechnical engineer.

### **1.3 SCOPE OF SERVICES**

The scope of services for this project included research of reference materials and field exploration. Section 5 contains a list of references consulted in the preparation of this report. The scope of services for this report was determined predominantly by client supplied standards.

Field exploration consisted of advancing one boring in the vicinity of the proposed tower base to a depth of 40 feet, or until auger refusal. Edge Consulting reviewed the boring logs, the recovered soil samples, and laboratory testing results (if any) to determine the engineering characteristics of the soils at or near the proposed tower location. This report summarizes the field exploration results and provides recommendations related to suitable foundation types and depths, allowable bearing pressure, and estimates of foundation settlement.

## **SECTION 2**

# **EXPLORATION RESULTS**

### **2.1 REFERENCE RESEARCH & BACKGROUND**

Review of United States Department of Agriculture NRCS (Natural Resource Conservation Service) Web Soil Survey for Grant County indicates natural site soils are classified as “DoC2” (Downs silt loam). These soil types are typically well drained soils that are gradually to somewhat steeply sloping and consist of loess. The soils are typically classified as CL on the Unified Soil Classification System. The risk of corrosion to uncoated steel is low and for concrete is moderate. Edge Consulting reviewed the “Thickness of Unconsolidated Material in Wisconsin” map prepared by the Geologic and Natural History Survey. This map indicates that the anticipated depth to bedrock is between 0-50 feet, with underlying bedrock consisting of dolomite with some limestone and shale of the Sinnipee Group from the Ordovician System.

### **2.2 TOPOGRAPHY**

The existing topography of the subject site is gradually sloping, with surface water generally flowing to the south. Existing slopes are approximately 2-5%. Site drainage is adequate, and no standing water was observed during drilling operations.

### **2.3 FIELD EXPLORATION**

One standard penetration test (SPT) soil boring was advanced to a depth of 13 feet below grade surface (bgs) due to auger refusal. Drilling was completed on March 31, 2022. The boring was advanced using a rotary drill rig. Representative soil samples were obtained using a standard 2-inch diameter split spoon sampler in general accordance with ASTM D1586 / D1586M-18, “Standard Test Method for Penetration Tests and Split-Barrel Sampling of Soils”. A description of this procedure is available in Appendix C of this report. Split spoon sampling was performed by collecting 18 inch samples at 2.5-foot intervals to a depth of 15 feet and 5 foot intervals thereafter.

The drill crew chief visually and manually classified samples in the field in accordance with ASTM D2487-17. The field personnel then collected representative soil samples from each split spoon and placed these samples in glass jars for further examination and verification of the field classification by a geotechnical engineer. The soil boring logs located in Appendix A contain pocket penetrometer readings, standard penetration measurements, soil classification information and other pertinent information.

Upon completion of drilling, the soil borings were abandoned in accordance with Chapter NR 141, Wisconsin Administrative Code.

## **2.4 SUBSURFACE CONDITIONS**

One soil boring was completed at the site. Boring B-1 was drilled as close to the proposed tower center location as was possible. Subsurface soils were noted to consist of 5 inches of topsoil followed by medium fine sandy, silty clay to 3 feet bgs. Weathered dolomite was then encountered to the end of boring and auger refusal at 13 feet bgs. The approximate location of the boring is depicted in Figure 3. The boring log is available in Appendix A.

## **2.5 BEDROCK**

Weathered dolomite was encountered at 3 feet bgs at the proposed tower center. This material was drillable until auger refusal was met at 13 feet bgs. A confirmation boring was drilled approximately 10 feet west of the proposed tower center and auger refusal was met at 12 feet bgs.

## **2.6 FROST DEPTH AND COVER**

According to the ANSI/TIA -222-G standards, frost depth for the area is expected to be 60 inches. It is recommended that all tower foundation elements, not bearing directly on solid rock or otherwise protected from frost, be founded at or below this depth to adequately protect against frost heave. Similarly, foundations for equipment buildings larger than 400 sq.ft. shall also extend below the frost line of the locality, be constructed on solid rock or be otherwise protected from frost in accordance with Section 1809.5 of the International Building Code (IBC).

## **2.7 WATER LEVEL OBSERVATIONS**

Groundwater was not encountered during this investigation.

## **2.8 LABORATORY TESTING**

Edge Consulting utilized a Bluelab® soil pH pen to obtain a pH reading of 7.4 from a collective soil sample obtained at the tower center from 3.5 to 5.0 feet bgs. The pH level was obtained in accordance with ASTM G51-95(2005) "Standard Test Method for Measuring pH of Soil for Use in Corrosion Testing".

A soil resistivity test was also completed on the same collective soil sample using a MC Miller soil box in conjunction with a Nilsson Model 400 Soil Resistance Meter to obtain a resistance reading of 2,300 OHM-cm. The soil resistance test was collected in accordance with ASTM G57-95a (Reapproved 2001) "Standard Test Method for Field Measurement of Soil Resistivity Using the Wenner Four-Electrode Method".

## **SECTION 3**

# **ANALYSIS & RECOMMENDATIONS**

### **3.1 FOUNDATIONS**

#### **3.1.1 Proposed Tower**

Based upon the information obtained during this investigation, Edge Consulting recommends the use of a standard spread footing (mat) to support the proposed tower. It is recommended that the tower foundation should bear on existing weathered dolomite at a minimum depth of 5 feet below existing grade surface. A complete listing of soil properties for use in foundation design is contained in Appendix B.

#### **3.1.2 Equipment Support Structure**

Edge Consulting recommends the use of conventional strip footings or an engineered floating slab for an equipment building foundation. Strip footings should have a minimum width of 12 inches. Strip footings for structures larger than 400 sq.ft. should extend to below the frost line of the locality or 48 inches below finished grade, whichever is greater. Footings bearing directly on solid rock above this depth or insulated by other means are also acceptable forms of frost protection.

If a floating slab foundation system is utilized, the slab should be designed in accordance with ACI 302.1R80 practice. Concrete floor slabs may be constructed on exposed subgrade or new compacted fill. In all cases, the exposed subgrade beneath new fill or the proposed floor slab should be compacted to 90% - 95% of the Modified Proctor maximum dry density (ASTM D1557). A minimum of 8 inches of compacted granular fill, or free draining gravel (ASTM C33, Size 57 concrete aggregate), should be located immediately beneath any floor slabs.

If an elevated equipment platform or building is to be utilized, Edge Consulting recommends supporting these structures with the use of individual drilled piers. Drilled piers for elevated equipment platforms and buildings should extend below the frost line of the locality or 48 inches below finished grade, whichever is greater.

### **3.2 SITE AND FOUNDATION DRAINAGE**

Positive site drainage should be provided to reduce infiltration of surface water into the backfills around the perimeters of tall proposed structures. All grades should slope away from these structures. Edge Consulting recommends that the top of tower foundation elements extend a minimum of 6 inches above the final site surface.

### **3.3 SITE PREPARATION & FILL RECOMMENDATIONS**

The following general site preparation and fill recommendations are provided for the development of this site.

- All vegetation, root-mat, topsoil, and any other soft or unsuitable material should be stripped from the areas of all proposed improvements. The removed material should be placed outside of any proposed improvement areas. These materials should not be utilized for backfill purposes.
- Removal of unsuitable fill material should be conducted within the footprint of the proposed improvements. Any construction debris should be removed from the site. The remaining material may be used as fill in other “non load bearing” areas of the site outside of the equipment pad, tower compound and road bed footprints.
- All areas requiring engineered fill should be brought up to grade. Engineered fill material should consist of clean well graded granular material containing less than 15% by weight passing the No. 200 sieve. This material should be placed in thin lifts not exceeding 8 inches in a loose thickness and compacted to 90% to 95% of the maximum dry density, as determined by ASTM D 1557, Modified Proctor test. Fill areas under footings should be extended from each side of the outermost location of the footing at a rate of 2/3 foot width for every 1 foot of fill depth. Fill placement activities should be performed in the presence of a qualified geotechnical engineer.
- Once final grade for footing placement has been established, density tests and/or examination by a geotechnical engineer should be performed in the footing trenches prior to footing placement to confirm that the material has achieved an adequate degree of compaction.
- Foundation and wall construction should follow in accordance with the structural engineers requirements.
- Once the prescribed curing time has been achieved, installation of any required perimeter drainage system and backfill may be initiated. Backfill of foundation walls may consist of onsite or imported granular material. Backfill should be installed in a maximum of 12 inch lifts and compacted to a 92% of the maximum dry density (D1557).

### **3.4 EXCAVATION SLOPE RECOMMENDATIONS**

It is expected that short term slopes of 1:1 can be maintained in the soils encountered at this site. However, construction practices should follow all federal, state and local regulations regarding safety standards for all excavation activities.

Construction site safety is the sole responsibility of the Contractor. Edge Consulting assumes no liability for Contractor's construction activities, construction site safety, or interpretation of information provided within this report. Such responsibility shall neither be implied nor inferred.



### **3.5 SOIL BEARING CAPACITY**

The recommended maximum presumptive net bearing capacity of weathered dolomite at depths greater than 3 feet bgs is 10,000 psf. Foundation systems designed for these capacities should experience a total settlement of less than 1 inch, with a differential settlement of less than half this amount. All bearing values should be considered allowable. A factor of safety of 2.0 has been assumed.

### **3.6 LATERAL EARTH PRESSURE**

Edge Consulting utilized Rankine methodology to determine the foundation earth pressure parameters. Recommended values for passive lateral earth pressure based on soil depth are available in Appendix B. All calculated values are considered ultimate. It is assumed that a minimum factor of safety of 2.0 will be incorporated at the time of foundation design.

### **3.7 FRICTIONAL SKIN RESISTANCE**

Included in Appendix B are recommended values for compressive frictional skin resistance for this site. These resistances are assumed to occur between concrete foundation elements and existing site soils. All calculated values are considered ultimate. It is assumed that a minimum factor of safety of 2.0 will be incorporated at the time of foundation design. For uplift and pull-out type calculations, it is further recommended that the uplift frictional skin resistance be considered as 2/3 of the listed compressive values.

### **3.8 SPECIAL DESIGN CONSIDERATIONS**

Based upon the proposed site grading plans for this site, the dimension from top of concrete at the tower anchor bolt setting to the foundation base should be a minimum of 5 feet to account for existing site contour, proposed filling, drainage and frost protection.

Weathered dolomite was encountered at 3 feet bgs followed by auger refusal at 13 feet bgs. The design of drilled pier foundations at this site is not recommended.

Shallow bedrock was encountered within the tower compound area. A site grounding system which utilizes grounding plates in lieu of ground rods may need to be considered.

### **3.9 SPECIAL CONSTRUCTION CONSIDERATIONS**

The contractor should review the proposed tower foundation design with the site construction plans prior to ordering foundation reinforcing steel. The tower foundation should be designed such that the top of concrete extends 6-inches above the proposed compound finished grade. In addition, the foundation base should extend to a minimum of 5.5 feet below existing grade. If the plans do not reflect this condition, please contact the project manager and foundation designer.

Weathered dolomite was encountered within 3 feet of ground surface. Based on the size of the tower it is anticipated that there will be a relatively large spread footing at the site. The depth to bedrock can vary widely across the width of the foundation. In addition, it is likely that relatively solid bedrock may be encountered above the depth of auger refusal. This is difficult to quantify without completing test pits at various locations. The contractor should anticipate significant difficulties in completing the necessary excavation to install the tower foundation, including the use of hydraulic hammering and/or blasting. If this is a major factor in the bidding process, the contractor may wish to complete several test pits within the boundaries of the proposed foundation to better determine the exact conditions that will be encountered prior to ordering the foundation steel.

Special precautions should be taken for earthwork during winter months. Footings or fills should not be placed on frozen soils. Exposed subgrade soil should be adequately protected with insulating blankets.

## **SECTION 4**

# **LIMITATIONS AND RESTRICTIONS**

This report has been prepared to aid in the evaluation of this property for the intended use described herein, and to assist in the design or planning of this project. In the event any changes in the design as outlined herein, or changes in the vertical position or horizontal location of the facility are planned, the conclusions and recommendations contained in this report shall not be considered valid unless such changes are reviewed by Edge Consulting Engineers, Inc.

The analysis and recommendations submitted in this report are our opinions based on the data obtained and subsurface conditions noted from the field investigation described at the locations indicated on the accompanying site plan. This report does not reflect any variations that may occur between, beyond, or below the depths of these test pits or borings. If variations then appear evident, it will be necessary for a re-evaluation of the recommendations of this report to be made after performing on-site observations during the construction period and noting the characteristics of any variations.

The soil report is only for the purposes stated in the contract and may not be sufficient to prepare an accurate bid.

Certain assumptions have been made regarding the foundation design for this site. Edge Consulting Engineers, Inc. should be given the opportunity to review the final foundation design to determine whether the final design necessitates any changes of the recommendations contained in this document. If Edge Consulting is not provided the opportunity for this review, we can assume no responsibility for the misinterpretation or misapplication of these recommendations or for their validity in the event changes have been made to the initial understanding of the project or design content.

There is the possibility that variations in soil conditions will be encountered during construction. In order to permit correlation between soil data in this report and the actual soil conditions encountered during construction, it is required that the soil engineer be retained to perform a review of the excavation prior to foundation placement. Edge Consulting assumes no responsibility for construction compliance with design concepts, specifications, or recommendations unless we have been retained to perform on-site review during the course of construction. Edge Consulting should be contacted immediately if conditions encountered are not consistent with those described.

This report was prepared in accordance with generally accepted soil and foundation engineering practices and makes no other warranties, either expressed or implied, as to the professional advice provided under the terms of the agreement between the Engineer and his client. This report has not been prepared for uses or parties other than those specifically named, or for uses or applications other than those enumerated herein. The report may contain insufficient or inaccurate information for other purposes, applications, building sites, or other uses.

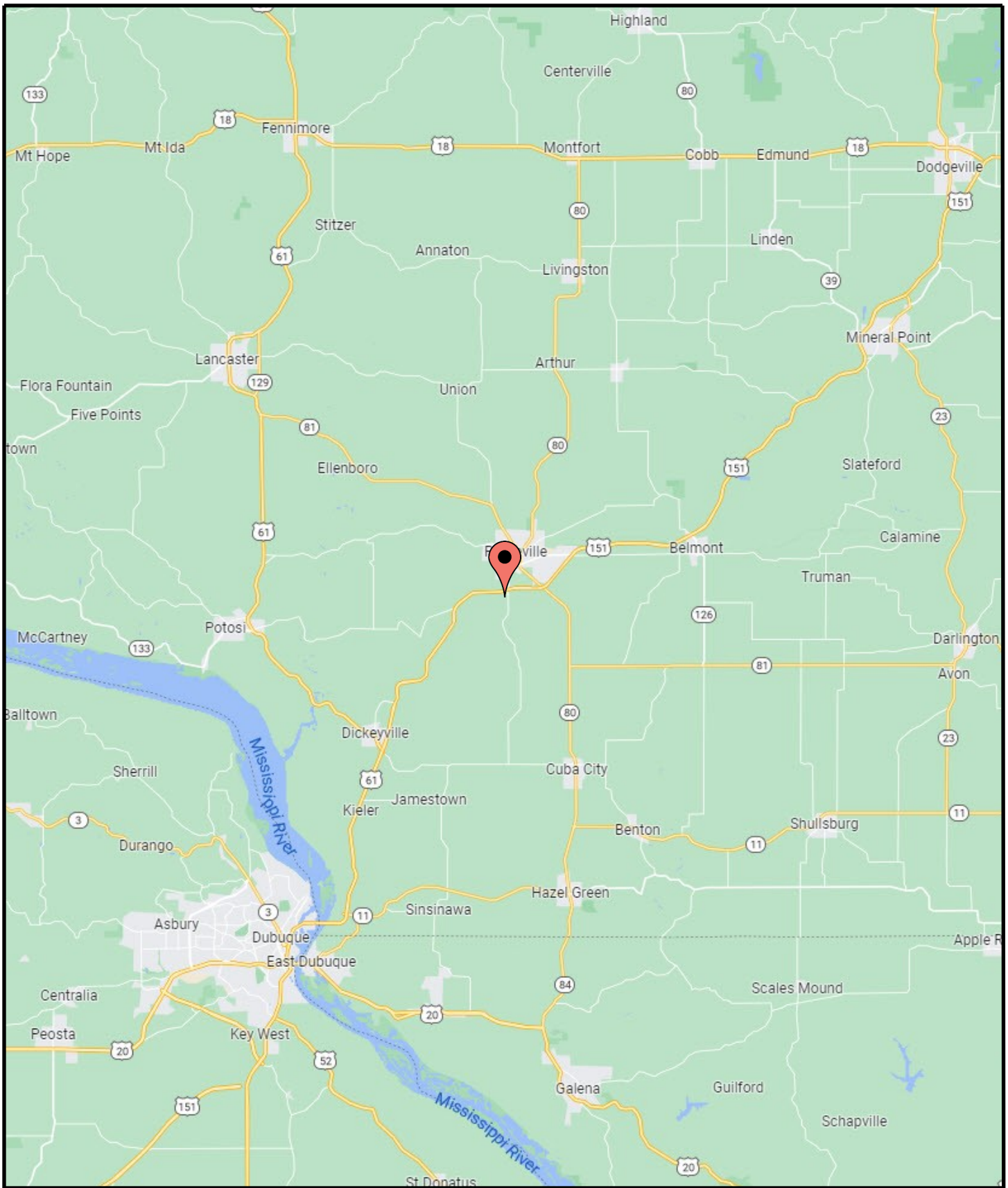
## SECTION 5

# REFERENCES

1. Das, Braja M., *Principles of Foundation Engineering*, 2nd. Edition, 1990.
2. Das, Braja M., *Principles of Geotechnical Engineering*, 2nd. Edition, 1990.
3. Das, Braja M., *Soil Mechanics Laboratory Manual*, 3rd. Edition, 1989.
4. Hadley, David W. and Pelham, James H., *Glacial Deposits of Wisconsin*, 1976.
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8. Reese, Lymon C., Ph.D., P.E. and Michael W. O'Neill Ph.D., P.E., Drilled Shafts: Construction Procedures and Design Methods, Publication Nos. FHWA-HI-88-042, ADSC-TL-4, August 1988.
9. Soil Survey Staff, Natural Resources Conservation Service, United States Department of Agriculture. Web Soil Survey. Available online at <http://websoilsurvey.gov/>.

# Figure 1

## Street Maps



**FIGURE #1: Regional Map**



**Edge**

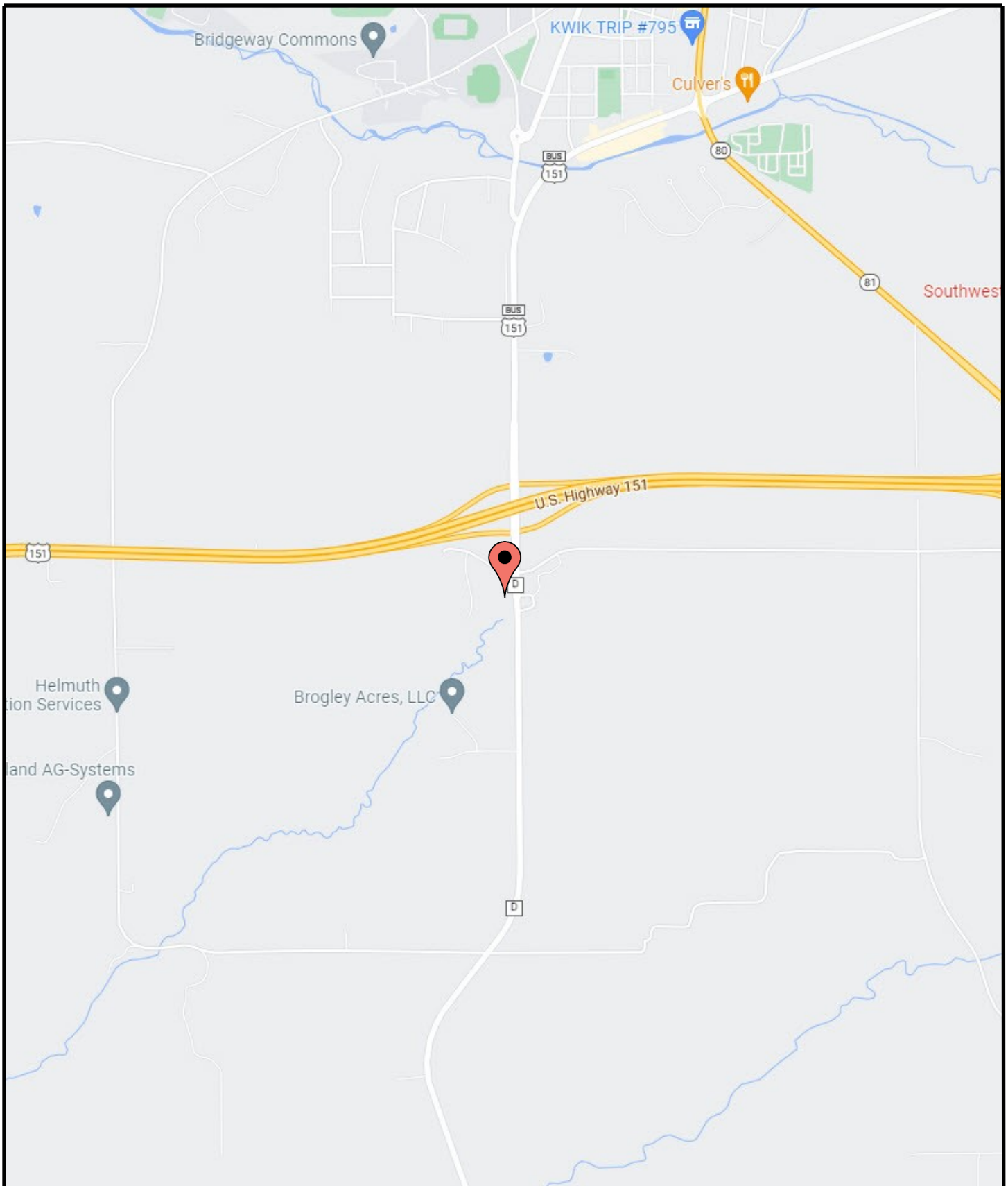
Consulting Engineers, Inc.

**Project Number:** #31990

**Project Name:** Grant County / Platteville GF2

**Project Address:** 5657 County Highway D, Platteville, Wisconsin 53818





**FIGURE #2: Street Map**



**Edge**

Consulting Engineers, Inc.

**Project Number:** #31990

**Project Name:** Grant County / Platteville GF2

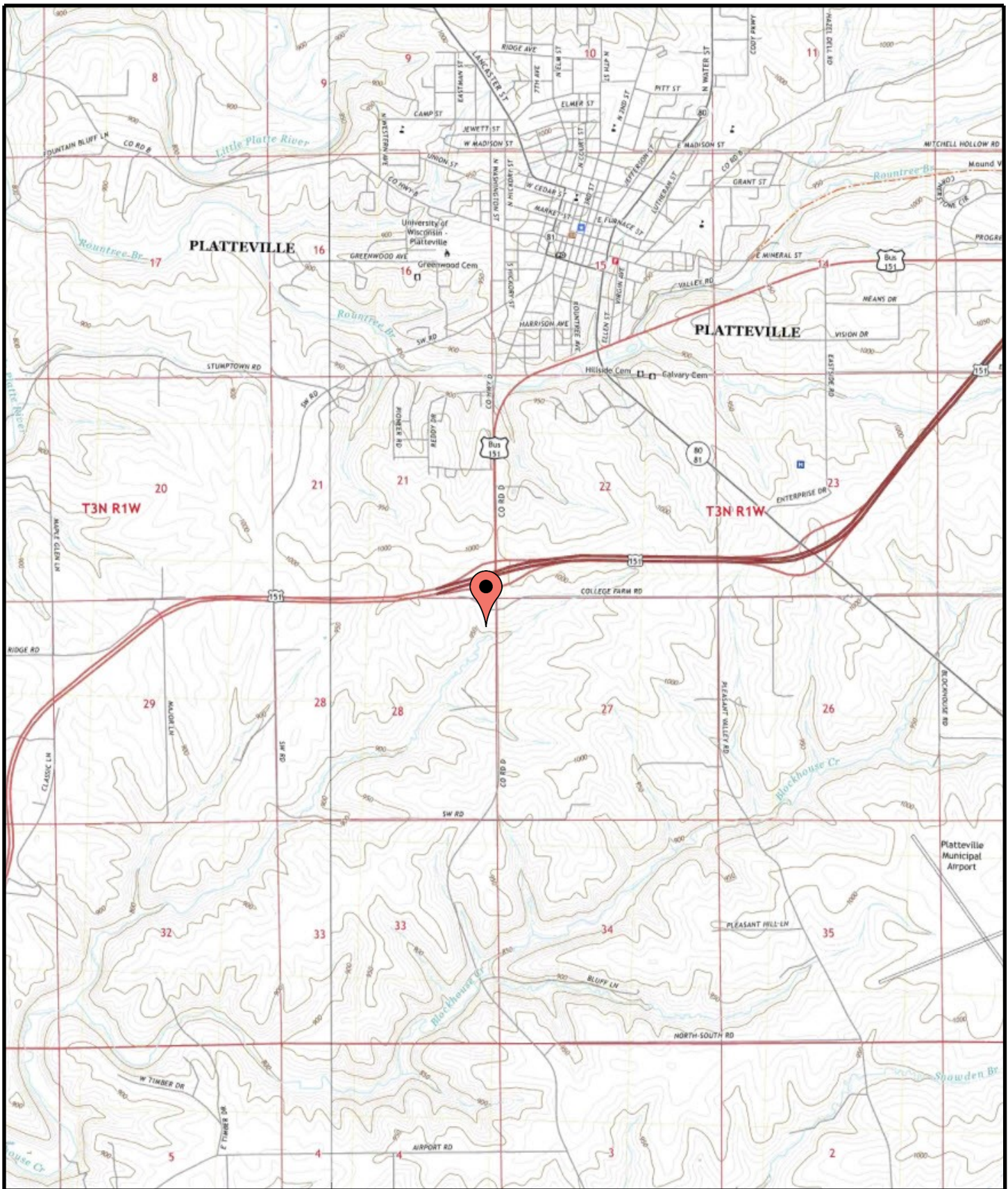
**Project Address:** 5657 County Highway D, Platteville, Wisconsin 53818



# **Figure 2**

USGS Topographic Quadrangle Map





**FIGURE #3: USGS 7.5-Minute Quadrangle**



**Edge**

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**Project Number:** #31990

**Project Name:** Grant County / Platteville GF2

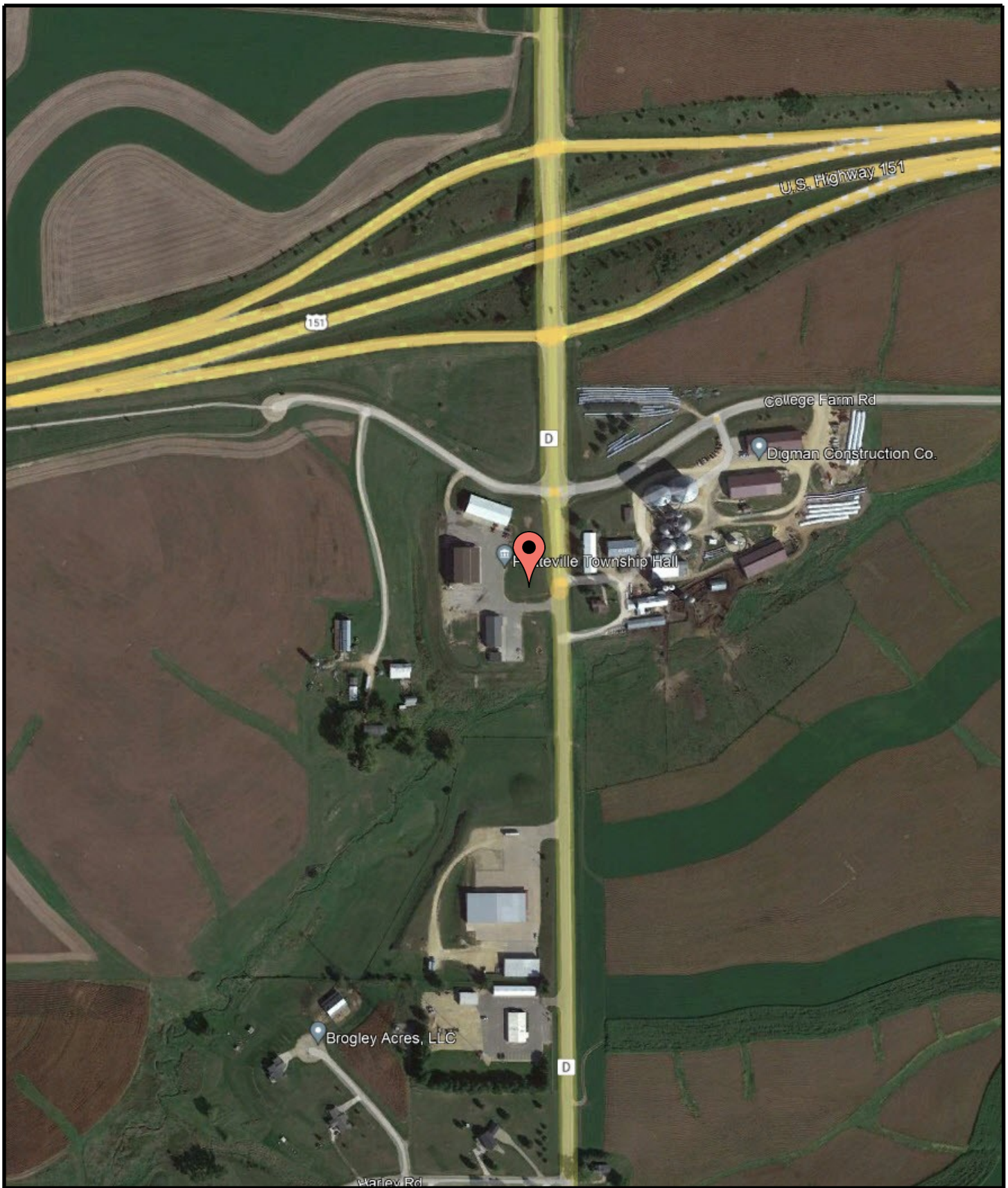
**Project Address:** 5657 County Highway D, Platteville, Wisconsin 53818



# **Figure 3**

## **Site Plan**





**FIGURE #4: Aerial/Satellite Plan View**



**Edge**

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**Project Number:** #31990

**Project Name:** Grant County / Platteville GF2

**Project Address:** 5657 County Highway D, Platteville, Wisconsin 53818





- A. 255' SELF-SUPPORT TOWER
- B. 75' x 75' FENCED COMPOUND
- C. 90' TOWER SETBACK RADIUS (FROM TOWER CENTER)



NORTH

RIGHT OF WAY

**COLLEGE FARM ROAD**

# С Т Н "Д"

SECTION LINE

EXISTING  
BUILDING

PARCEL #: 050-00808-0030  
OWNER: GRANT COUNTY HWY DEPT.

EXISTING  
BUILDING

Boring B-1

EXISTING  
BUILDING

EXISTING TRANSFORMER 6-28-T2

EXISTING TRANSFORMER

— SINGLE ELECTRIC METER PEDESTAL

PARCEL #: 050-00808-0040  
OWNER: PLATTEVILLE TOWNSHIP

1. NORTH ARROW SHOWN AS APPROXIMATE.
2. AERIAL IMAGERY PER:  
GRANT COUNTY GIS MAPPING DATA
3. PROPERTY LINES SHOWN PER SURVEY COMPLETED BY:  
MERIDIAN SURVEYING, LLC

I:\31900\31990\Design\CAD\PR2\Plot\C-101.dgn

CONSULTANT:



CLIENT:



# OVERALL SITE PLAN

SUBMITTAL:

[illegible]

CHECKED	
---------	--

A 10

PLOT	
------	--

3/22/2022

PROJECT

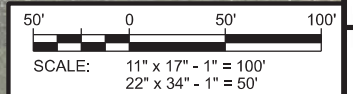
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SET

550

SHEET

C-101





# **Figure 4**

## **Site Photographs**



**VIEW NORTH TOWARD PROJECT LOCATION**



**VIEW SOUTH TOWARD PROJECT LOCATION**

**FIGURE #5: Project Location Photographs**



**Edge**

Consulting Engineers, Inc.

**Project Number:** #31990

**Project Name:** Grant County / Platteville GF2

**Project Address:** 5657 County Highway D, Platteville, Wisconsin 53818

**Photograph Date:** March 28, 2022





**VIEW EAST TOWARD PROJECT LOCATION**



**VIEW WEST TOWARD PROJECT LOCATION**

**FIGURE #5: Project Location Photographs**



**Edge**

Consulting Engineers, Inc.

**Project Number:** #31990

**Project Name:** Grant County / Platteville GF2

**Project Address:** 5657 County Highway D, Platteville, Wisconsin 53818

**Photograph Date:** March 28, 2022

# **Appendix A**

## **Soil Boring Logs**



**Project: Grant County (Platteville GF2)**  
**Project Location: Platteville, Wisconsin**  
**Project Number: 31990**

# Log of Boring B-1

Sheet 1 of 1

Date(s) Drilled	<b>March 31, 2022</b>	Logged By	<b>CRJ</b>	Checked By	<b>Tim</b>
Drilling Method	<b>Hollow Stem Auger</b>	Drill Bit Size/Type	<b>2 inch Split Spoon</b>	Total Depth of Borehole	<b>13 feet bgs</b>
Drill Rig Type	<b>Geoprobe 7822DT</b>	Drilling Contractor	<b>Soil Essentials</b>	Approximate Surface Elevation	<b>965 feet MSL</b>
Groundwater Level and Date Measured	<b>Not Encountered ATD</b>	Sampling Method(s)	<b>SPT, Grab</b>	Hammer Data	<b>140 lb, 30 in drop, rope &amp; cathead</b>
Borehole Backfill	<b>Bentonite/Cuttings</b>	Location	<b>Tower Center</b>		

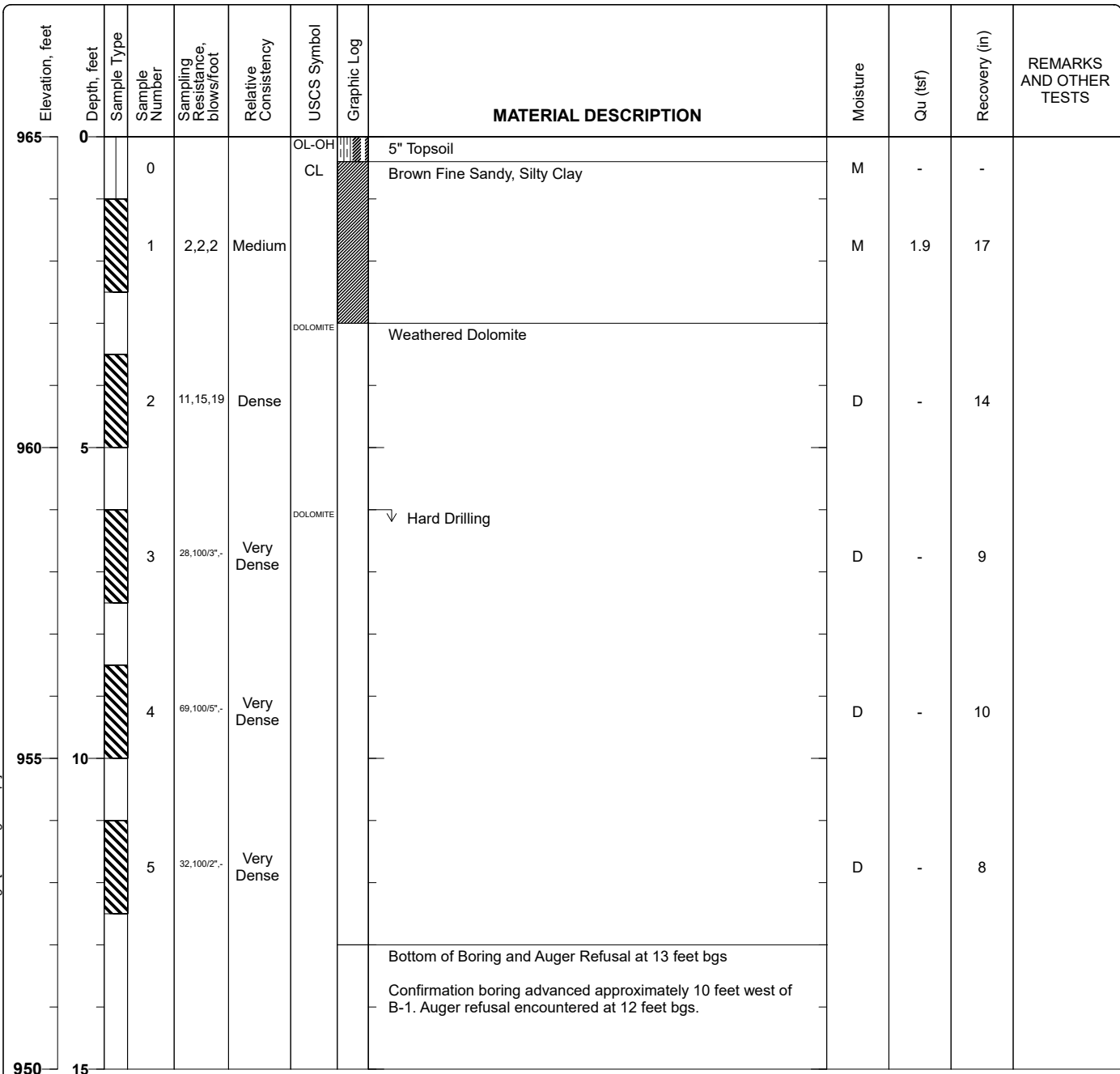


Figure 1

**Project: Grant County (Platteville GF2)**  
**Project Location: Platteville, Wisconsin**  
**Project Number: 31990**

## Key to Log of Boring

Sheet 1 of 1

Elevation, feet	Depth, feet	Sample Type	Sample Number	Sampling Resistance, blows/foot	Relative Consistency	USCS Symbol	Graphic Log	MATERIAL DESCRIPTION	Moisture	Qu (tsf)	Recovery (in)	REMARKS AND OTHER TESTS
-----------------	-------------	-------------	---------------	---------------------------------	----------------------	-------------	-------------	----------------------	----------	----------	---------------	-------------------------

1 2 3 4 5 6 7 8 9 10 11 12 13

### COLUMN DESCRIPTIONS

































- 1 **Elevation, feet:** Elevation (MSL, feet)
- 2 **Depth, feet:** Depth in feet below the ground surface.
- 3 **Sample Type:** Type of soil sample collected at the depth interval shown.
- 4 **Sample Number:** Sample identification number.
- 5 **Sampling Resistance, blows/foot:** Number of blows to advance driven sampler foot (or distance shown) beyond seating interval using the hammer identified on the boring log.
- 6 **Relative Consistency:** Relative consistency of the subsurface material.
- 7 **USCS Symbol:** USCS symbol of the subsurface material.
- 8 **Graphic Log:** Graphic depiction of the subsurface material encountered.
- 9 **MATERIAL DESCRIPTION:** Description of material encountered. May include consistency, moisture, color, and other descriptive text.
- 10 **Moisture:** Water content of the soil sample, expressed as percentage of dry weight of sample.
- 11 **Qu (tsf):** Dry weight per unit volume of soil sample measured in laboratory, in pounds per cubic foot.
- 12 **Recovery (in):** The percent fines (soil passing the No. 200 Sieve) in the sample. WA indicates a Wash Sieve, SA indicates a Sieve Analysis.
- 13 **REMARKS AND OTHER TESTS:** Comments and observations regarding drilling or sampling made by driller or field personnel.

### FIELD AND LABORATORY TEST ABBREVIATIONS









**CHEM:** Chemical tests to assess corrosivity  
**COMP:** Compaction test  
**CONS:** One-dimensional consolidation test  
**LL:** Liquid Limit, percent  
**PI:** Plasticity Index, percent

**SA:** Sieve analysis (percent passing No. 200 Sieve)  
**UC:** Unconfined compressive strength test, Qu, in ksf  
**WA:** Wash sieve (percent passing No. 200 Sieve)






### TYPICAL MATERIAL GRAPHIC SYMBOLS

 Well graded GRAVEL (GW)	 Well graded SAND with Clay (SW-SC)	 SILTY CLAY (CL-ML)
 Poorly graded GRAVEL (GP)	 Poorly graded SAND with Silt (SP-SM)	 Lean CLAY/PEAT (CL-OL)
 Well graded GRAVEL with Silt (GW-GM)	 Poorly graded SAND with Clay (SP-SC)	 Fat CLAY/SILT (CH-MH)
 Well graded GRAVEL with Clay (GW-GC)	 Silty SAND (SM)	 Fat CLAY/PEAT (CH-OH)
 Poorly graded GRAVEL with Silt (GP-GM)	 Clayey SAND (SC)	 Silty SAND to Sandy SILT (SM-ML)
 Poorly graded GRAVEL with Clay (GP-GC)	 SILT, SILT w/SAND, SANDY SILT (ML)	 Silty SAND to Sandy SILT (SM-MH)
 Silty GRAVEL (GM)	 Lean CLAY, CLAY w/SAND, SANDY CLAY (CL)	 Clayey SAND to Sandy CLAY (SC-CL)
 Clayey GRAVEL (GC)	 SILT, SILT w/SAND, SANDY SILT (MH)	 Clayey SAND to Sandy CLAY (SC-CH)
 Well graded SAND (SW)	 Fat CLAY, CLAY w/SAND, SANDY CLAY (CH)	 SILT to CLAY (CL/ML)
 Poorly graded SAND (SP)	 SILT, SILT with SAND, SANDY SILT (ML-MH)	 Silty to Clayey SAND (SC/SM)
 Well graded SAND with Silt (SW-SM)	 Lean-Fat CLAY, CLAY w/SAND, SANDY CLAY (CL-CH)	

### TYPICAL SAMPLER GRAPHIC SYMBOLS

 2-inch-OD unlined split spoon (SPT)	 Shelby Tube (Thin-walled, fixed head)	 Pitcher Sample
 2.5-inch-OD Modified California w/ brass liners	 Grab Sample	 Other sampler
 3-inch-OD California w/ brass rings	 Bulk Sample	

### OTHER GRAPHIC SYMBOLS

 Water level (at time of drilling, ATD)
 Water level (after waiting a given time)
 Minor change in material properties within a stratum
 Inferred or gradational contact between strata
 Queried contact between strata

### GENERAL NOTES

- Soil classifications are based on the Unified Soil Classification System. Descriptions and stratum lines are interpretive, and actual lithologic changes may be gradual. Field descriptions may have been modified to reflect results of lab tests.
- Descriptions on these logs apply only at the specific boring locations and at the time the borings were advanced. They are not warranted to be representative of subsurface conditions at other locations or times.

# **Appendix B**

## **Soil Properties, Calculations**

## SOIL PROFILE

### Notes:

Groundwater not encountered  
Frost depth is at 5' BGS  
k, E50 values are for Lpile use only

Medium Sandy Silty Clay (CL) (0 to 3 feet BGS)	
$\gamma =$	110 pcf
$\Phi =$	0 °
Cu =	1,000 psf
E50 =	0.01
k =	100 pci
Bc =	2,000 psf

Weathered Dolomite (>3 feet BGS)	
$\gamma =$	145 pcf
$\Phi =$	41 °
Cu =	0 psf
E50 =	N/A
k =	225 pci
Bc =	10,000 psf

## SOIL PROPERTIES

Depth (feet)	Unit Weight (pcf)	Friction Angle (°)	Cohesion (psf)	Effective Stress (psf)	Passive Pressure (psf)	Skin Friction Resistance (psf)
0	110	0°	1,000	0	0	400
1	110	0°	1,000	110	2,110	400
2	110	0°	1,000	220	2,220	400
3	110	0°	1,000	330	2,330	400
3	145	41°	0	365	1,757	462
4	145	41°	0	475	2,287	584
5	145	41°	0	620	2,985	743
6	145	41°	0	765	3,683	895
7	145	41°	0	910	4,382	1,040
8	145	41°	0	1,055	5,080	1,180
9	145	41°	0	1,200	5,778	1,314
10	145	41°	0	1,345	6,476	1,443
11	145	41°	0	1,490	7,174	1,568
12	145	41°	0	1,635	7,872	1,688
13	145	41°	0	1,780	8,571	1,804

# **Appendix C**

## **Classification of Soils for Engineering Purposes**



# UNIFIED SOIL CLASSIFICATION SYSTEM

Major divisions		Group symbols		Typical names		Laboratory classification criteria		
Coarse-grained soils (More than half of material is larger than No. 200 sieve size)	Gravels (More than half of coarse fraction larger than No. 4 sieve size)	Clean gravels (Little or no fines)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines		$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3		
			GP	Poorly graded gravels, gravel-sand mixtures, little or no fines		Not meeting all gradation requirements for GW		
		Gravels with fines (Appreciable amount of fines)	GM	d	Silty gravels, gravel-sand-silt mixtures		Atterberg limits below "A" line or P.I. less than 4	
				u				
		GC	Clayey gravels, gravel-sand-clay mixtures		Atterberg limits above "A" line with P.I. greater than 7			
	Sands (More than half of coarse fraction is smaller than No. 4 sieve size)	Clean sands (Little or no fines)	SW	Well-graded sands, gravelly sands, little or no fines		$C_u = \frac{D_{60}}{D_{10}}$ greater than 6; $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3		
			SP	Poorly graded sands, gravelly sands, little or no fines		Not meeting all gradation requirements for SW		
		Sands with fines (Appreciable amount of fines)	SM	d	Silty sands, sand-silt mixtures		Atterberg limits below "A" line or P.I. less than 4	
				u				
		SC	Clayey sands, sand-clay mixtures		Atterberg limits above "A" line with P.I. greater than 7			
Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows: Less than 5 per cent . . . . . GW, GP, SW, SP More than 12 per cent . . . . . GM, GC, SM, SC 5 to 12 per cent . . . . . <i>Borderline</i> cases requiring dual symbols								
Fine-grained soils (More than half of material is smaller than No. 200 sieve)	Silts and clays (Liquid limit less than 50)	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity		<p>For classification of fine-grained soils and fine fraction of coarse-grained soils. Atterberg Limits plotting in hatched area are <i>borderline</i> classifications requiring use of dual symbols. Equation of A-line: <math>PI = 0.73 (LL - 20)</math></p>			
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays					
		OL	Organic silts and organic silty clays of low plasticity					
	Silts and clays (Liquid limit greater than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts					
		CH	Inorganic clays of high plasticity, fat clays					
		OH	Organic clays of medium to high plasticity, organic silts					
	Highly organic soils	Pt	Peat and other highly organic soils					



### Penetration Testing Procedure - "N" Values

The penetration testing procedure used for this project followed the requirements of ASTM Specification D 1586-67, "Standard Method for Penetration Tests and Split-Barrel Sampling of Soils". This procedure involves driving a 2-inch OD standard split spoon sampler 18 inches with a 140-pound hammer free falling a distance of 30 inches. The number of blows required to drive the sampler the final foot was recorded as the Standard "N" Penetration. This N-value is used by Soils Engineers to estimate the strength and compressibility of the soil. After driving, the sampler was returned to the surface and opened. The length of sample (recovery) was measured and the soil was preliminarily classified according to type by a Soils Technician. A representative portion of each sample was then sealed in a glass jar, labeled, and returned to our office for further examination and testing.